

## Real Data from Pilot Seals from Repositories in Salt and their Utilization in a Safety Case

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Dr. Nina Müller-Hoeppe (BGE TECHNOLOGY GmbH)



## **Motivation**



- Safe confinement of radioactive waste in a confinement providing rock zone (CRZ)
- Recovery of the intact salt barrier by compaction of crushed salt
- During the time period until recovery is finalized geotechnical barriers guarantee safe confinement
- Redundancy and spatial separation of geotechnical barriers -> shaft and drift seals



Compaction of crushed salt depending on operating conditions of a salt repository (extrapolated numerically)



## Example Preliminary safety analysis of the Gorleben site







## Focus on drift seals – Salt/Sorel concrete



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Mixtures (building site)	Binding agents	Mixing liquid	Aggregate	Additive
Salt concrete Type Asse (Asse)	Cement (380 kg/m <sup>3</sup> )	NaCl- solution (198 kg/m <sup>3</sup> )	Crushed salt (1,496 kg/m <sup>3</sup> )	-
Salt concrete M2 (ERAM)	Cement (328 kg/m <sup>3</sup> )	Water (267 kg/m <sup>3</sup> )	Crushed salt (1,077 kg/m <sup>3</sup> )	Coal fly ash (328 kg/m <sup>3</sup> )
Sorel concrete 29.6 A2 (Asse)	MgO (120 kg/m <sup>3</sup> ) & calcinated dolomite (225 kg/m <sup>3</sup> )	MgCl <sub>2</sub> -rich solution (490 kg/m <sup>3</sup> )	Crushed salt (900 kg/m <sup>3</sup> )	Slate powder (255 kg/m <sup>3</sup> )
Sorel concrete A1 (Asse)	MgO (218 kg/m <sup>3</sup> )	MgCl <sub>2</sub> -rich solution (485 kg/m <sup>3</sup> )	Crushed salt (1,237 kg/m <sup>3</sup> )	-





Basically, a drift seal consists of three elements acting in parallel:

- > The sealing body made of salt- or sorelconcrete
- The EDZ in salt close to the drift contour
- The contact zone between sealing body and EDZ







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## Tests without hydromechanical loading





## **Motivation**

In the case of concrete structures constituting a barrier against water contaminants technical guidelines require investigations of contact zones at comparable structures as they may act as preferential pathways

- The Asse-seal made of salt concrete (abandoned in situ research project) was available as a comparable structure to the conceptual planning of ERAM drift seals
- > The following in situ investigations were performed:
  - Permeability measurements in the contact zone and for comparison in the sealing body and the (former) EDZ as well
  - Stress measurements in order to transfer the stress state to ERAM in situ conditions
  - Ultrasonic measurements in order to detect imperfections of the overall contact zone



## **Results of permeability measurements**





For evaluation of the seal's hydraulic resistance 34 measurements from boreholes were available



## **Results of ultrasonic measurements**



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Photo documentation from the building phase showed that high permeability values in the roof resulted from a hand filled gap  $\rightarrow$  Not representative!

#### Lesson learnt

Inclining the roof of salt contour in order to improve future drift seal constructions



## Evaluation of permeability values using Eurocode "Geotechnical Design"



Derivation of "cautious estimates" for permeabilities of sealing body, contact zone and, excavation damaged zone/rock salt

Element	5%-Fractile [m²]	Upper confidence limit of mean value [m <sup>2</sup> ]	Direct design value with p <sub>f</sub> < 0.1% [m <sup>2</sup> ]	Mean value [m²]
Sealing body	3·10 <sup>-22</sup>	5·10 <sup>-23</sup>	3·10 <sup>-21</sup>	3·10 <sup>-23</sup>
Contact zone except roof	4·10 <sup>-22</sup>	7·10 <sup>-23</sup>	1·10 <sup>-20</sup>	1·10 <sup>-23</sup>
Contact zone roof	6.2·10 <sup>-13</sup>	4.7·10 <sup>-13</sup>	6.3·10 <sup>-12</sup>	4.6·10 <sup>-14</sup>
Former EDZ	2·10 <sup>-21</sup>	1·10 <sup>-22</sup>	8·10 <sup>-20</sup>	2·10 <sup>-23</sup>

This state of knowledge was applied to the pilot seal BW-K2C-750-1 in the Asse mine in order rate the permeability of the sealing body made of Sorel concrete A1.



# Evaluation of permeability values using a geostatistical approach



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- Sampling for spatial variability
  - Each sample is assumed as representative for the whole seal structure
  - In reality, though, all samples come from one and the same seal and represent hydraulic conductivity at different locations of this seal
- Question
  - How to "upscale" these data (packer scale 10 cm) in order to derive conclusions for the whole seal?

## Geostatistical approach

- Consider "true" seal (the one the data come from) as one realisation of a locationdependent random variable ("random function")
- Derive conclusions about pdf and spatial behaviour of the random function from the data ("variography", central: "assumption of stationarity")
- Sample realisations of the so described random variable (one realisation corresponds with one "possible seal")
- Calculate hydraulic flow (for given pressure gradient) for each realisation and calculate effective conductivity (permeability)
- Perform statistics for these effective conductivities (permeabilities)





### **Example - One realization**



log<sub>10</sub>(permeability [m<sup>2</sup>])





## **Effective conductivity**



Statistics for effective conductivity (different model assumptions)



## Conclusion

 For a seal which is correctly described by the underlying assumptions, the effective conductivity is with a likelihood of 1/10,000 smaller than 1.3·10<sup>-16</sup> m/s (permeability 1.3 ·10<sup>-23</sup> m<sup>2</sup>, statistical confidence 95 %).





- Integral permeability using geostatistical methods
- For a seal which is correctly described by the underlying assumptions, the effective conductivity is with a likelihood of 1/10.000 smaller than 1,3.10<sup>-23</sup> m<sup>2</sup> (statistical confidence 95 %).
- Integral permeability using European Standard of Geotechnical Design (EN 1997)
- Upper confidence limit of integral permeability 6.7.10<sup>-23</sup> m<sup>2</sup>
- Mean value of integral permeability 2.6.10-23 m<sup>2</sup>
- The upper confidence limit of the mean value (reliable mean value) according European Standard of Geotechnical Design can reliably be applied in practice



## Pilot seal BW-K2C-750-1 – Sorel concrete



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Reliable mean value of sealing body (upper limit) 4.5E-18 m<sup>2</sup> ~ 5E-18 m<sup>2</sup>

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1.00E-18 1.20E-19 7.00E-18 2.20E-18

Sealing body

Measured permeability values

[m<sup>2</sup>]

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1.20E-18

3.00E-18

2.70E-17

1.80E-18

4.00E-18

1.20E-18

3.00E-18



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# Tests <u>with</u> hydromechanical loading via pressure chamber





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## Finally: Concreting of the core barrier (complete)





## Pilot seal A2 (PSB A2)



- Construction features (2002-2003)
  - Based on mining experience the EDZ was removed shortly before concreting of abutments and core barrier
  - The roof was inclined in order to achieve positive locking to sealing body
  - The sealing body was made of a very low permeable but soft building material (Sorel concrete 29.6 A2) in order to avoid potentially crack-inducing stress peaks
  - Installation of measuring devices temperature and total pressure



## **PSB A2 – Construction phase**



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PSB A2 center – temperatures versus time



## **PSB A2** – Pressure build-up phase



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## **PSB A2 – Integral permeability**



![](_page_20_Picture_2.jpeg)

![](_page_20_Figure_3.jpeg)

## Lessons learnt from PSB A2

- Removal of EDZ according to mining experience is not sufficient
- $\rightarrow$  Prospective removal of EDZ is based on measured data
- Selection of "soft" but low permeable building material is not favourable
- $\rightarrow$  EDZ acts as preferential pathway
- $\rightarrow$  Use of "stiff" and low permeable building material

![](_page_20_Picture_11.jpeg)

## Pilot seal A1 (PSB A1)

![](_page_21_Picture_1.jpeg)

- Improved construction features (2006)
  - Removal of EDZ based on measured values
  - Selection of "stiff" building material for sealing body
  - Installation of measuring devices temperature, total pressure, pore pressure

![](_page_21_Figure_6.jpeg)

## Pilot seal A1 (PSB A1)

![](_page_22_Picture_1.jpeg)

![](_page_22_Figure_2.jpeg)

![](_page_22_Figure_3.jpeg)

Pore pressure remained well below total pressure

- $\rightarrow$  No pressure decrease with time
- → PSB A1 is lower permeable than PSB A2

## Lesson learnt from PSB A1

- Use of "stiff" building material accelerates rock pressure build-up
- $\rightarrow$  Total pressure well above pore pressure prevents pathways via EdZ

![](_page_22_Picture_11.jpeg)

## ERAM pilot seal "Abdichtbauwerk im Steinsalz" – salt concrete

![](_page_23_Picture_1.jpeg)

- Special test specification (2010)
  - Keep brine pressure below total (radial) pressure

![](_page_23_Figure_4.jpeg)

![](_page_23_Picture_6.jpeg)

## Abdichtbauwerk im Steinsalz – Integral permeability

![](_page_24_Picture_1.jpeg)

![](_page_24_Figure_2.jpeg)

 $\rightarrow$  No seeping brine detected at drift face of seal

 $\rightarrow$  Calculation of low integral permeability based on outflow rate

## Conclusion

• Observing strictly lessons learnt enables to build tight drift seals in rock salt.

![](_page_24_Picture_8.jpeg)

![](_page_25_Picture_1.jpeg)

- Summary of lessons learnt
  - Removal of EDZ must be based on measuring results
  - Inclining the salt contour in the roof to achieve positive fit of salt contour and sealing body
  - Adequate "stiffness" of sealing body accelerates pressure build-up in the EdZ
  - For prognosis of rock pressure build-up stress state and convergence rate of drift seal location must be known, reliably
  - Selection of high converging drift seal locations accelerates safe confinemement of radioactive waste
  - Measures (e.g. pore storages within the sealing system) slowing down fluid pressure build-up at drift seal's location keep pore pressure below rock pressure in EdZ

## **Conclusion**

It is demonstrated that drift seals can be erected as a component of a sealing system that guarantees safe confinement of radioactive waste in rock salt.

![](_page_25_Picture_12.jpeg)

![](_page_26_Picture_1.jpeg)

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![](_page_26_Picture_7.jpeg)

![](_page_27_Picture_0.jpeg)

![](_page_27_Picture_2.jpeg)

![](_page_27_Picture_4.jpeg)